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Geotechnical & Environmental Engineering

Construction Materials Engineering & Testing

Laboratory Testing of Soils, Concrete & Asphalt

Geo-Environmental Drilling & Sampling

Preliminary Geotechnical Report

Ottawa Industrial Park - Unit 3

US Route 6, East of IL Route 71

Ottawa, Illinois

City of Ottawa

GEOTECHNICAL GROUP

November 21, 2025

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Mr. David A. Noble, P.E.
City of Ottawa
301 W. Madison St.
Ottawa, IL 61350

RE: L-99,961B
Ottawa Industrial Park - Unit 3
US Route 6, East of IL Route 71
Ottawa, Illinois

Dear Mr. Noble:

This report presents results of a preliminary soils exploration performed for Unit 3 of the Ottawa Industrial Park. These geotechnical engineering services are being provided in accordance with TSC Proposal No. 76,557 dated October 14, 2025 and the attached General Conditions, incorporated herein by reference. The preliminary soils exploration for Unit 3 was performed in conjunction with preliminary subsurface explorations for Units 2, 4 and 5 of the industrial park. As requested, separate geotechnical reports are being prepared for each unit.

Unit 3 of the Ottawa Industrial Park is located on the south side of US Route 6 about 1 mile east of IL Route 71, near the east end of the City of Ottawa, Illinois. It is bordered along the south side by the CSX Railroad. A narrow detention pond is present along the south end of the site. The Unit 3 parcel otherwise mostly consists of farmland. Based on aerial photos of the site (Google Earth), poor-drainage/marshy areas are present at the site especially in the northern portion of it. The site is relatively flat, with the ground surface elevations at the boring locations varying by about 3½ feet.

It is assumed that the development will include the eventual construction of 1 to 2-story, slab-on-grade light industrial structures. In order to develop the recommendations contained in this report, it has been assumed that column and wall loads will not exceed 150 kips and 8 kips per lineal foot (klf), respectively. No building plans and grading information were available at the time this report was prepared, e.g. top of finished floor (FF) for the various buildings have not been set. It is understood that the narrow detention pond present along the south end of the site may be expanded.

Field Investigation and Laboratory Testing

Thirteen (13) soil borings (Nos. SB-1 thru SB-8 & B-16 thru B-20) were previously performed at the site by Midwest Testing Services, Inc. (MTS) in February 2012 and June 2019. They were typically extended to depths of 3 to 8.5 feet below existing grade, with two (2) of the borings (B-19 & B-20) being made 20 feet deep.

As directed, five (5) additional soil borings (B3-1 through B3-5) were drilled by TSC in October 2025 as part of the preliminary subsurface exploration for Unit 3. The boring locations were selected and laid out in the field by others. The boring locations and ground surface elevations at them were provided to

TSC on a drawing labeled "Soil Boring Location Map" (dated October 28, 2025 and prepared by Etscheid, Duttlinger & Associates, Inc.). Reference is made to this Boring Location Plan in the Appendix for the drilling layout, which also includes the borings performed for Units 2, 4 and 5 as well as the previous MTS borings. The ground surface elevations at the TSC borings have been rounded to nearest 0.1 foot on the boring logs.

The most recent borings (B3-1 through B3-5) were all planned to be extended to a depth of 16 feet below existing grade, or refusal (or hard drilling) on bedrock. However, they were extend to a depth of 10 feet due to hard drilling (very slow progress) in apparent bedrock. The borings were drilled and samples tested according to currently recommended American Society for Testing and Materials specifications. Soil sampling was performed at 2½-foot in conjunction with the Standard Penetration Test (SPT), for which driving resistance to a 2" split-spoon sampler (N-value in blows per foot) provides an indication of the relative density of granular materials and consistency of cohesive soils. Water level readings were taken during and upon completion of drilling operations.

All soil samples were examined in the laboratory to verify field descriptions and to classify them in accordance with the Unified Soil Classification System. Laboratory testing included water (moisture) content determinations for all cohesive and intermediate (silt or loamy) soil types. An estimate of unconfined compressive strength was obtained for all cohesive soils using a calibrated pocket penetrometer. An organic matter content determination by the loss-on-ignition (LOI) method was also performed on a sample of surficial topsoil materials collected in Boring B3-1.

Reference is made to the boring logs in the Appendix which indicate subsurface stratigraphy, soil/rock descriptions, results of field and laboratory tests, as well as water level observations. The MTS boring logs and definitions of descriptive terminology for the TSC borings are also included. While strata changes are shown as a definite line on the boring logs, the actual transition between soil layers will probably be more gradual. It should be noted that in the absence of foreign substances it is often very difficult to distinguish fill materials from native soil samples.

Fluctuations in the groundwater levels may also occur due to variations in precipitation (short-term and seasonal) as well as rises or drops in river, creek, pond or other nearby surface water features, i.e. water levels at a future date may be higher or lower than those recorded at the time of drilling.

Discussion of Test Data

Approximately 8 to 15 of clayey topsoil was typically encounted at the surface of the borings. The exceptions included TSC Borings B3-1 and B3-3 (and also apparently MTS Borings B-19 and B-20) where the topsoil layer was about 3 feet deep. The samples of the thicker topsoil deposits at Borings B3-1 and B3-3 exhibited water contents of 28 to 31 percent. An LOI test performed on the sample of the surficial topsoil collected in Borings B3-1 also revealed an organic matter content of 6.4 percent.

The topsoil layer was underlain by native silty clay deposits ranging from medium stiff to hard in consistency at TSC Borings B3-2, B3-4 and B3-5 and at the majority of the MTS Borings, extending to depths ranging from about 3 to 6 feet below existing grade. The samples of these cohesive soils exhibited variable unconfined compressive strengths and water contents ranging from 0.7 to 4.25 tons per square foot (tsf) and 25 to 48 percent, respectively.

A soft organic silty clay deposit was encountered underlying the above described native silty clay soils at a depth of 3 feet below existing grade in Boring B3-2, extending to a depth of 5.5 feet. The sample of the soft organic deposit exhibited a high water content of 64 percent, indicative of a compressible nature.

The topsoil layer in B3-1, the organic silty clay deposit in B3-2 and the native silty clay layer in B3-5 were underlain by medium dense to very dense silt, clayey silt and sandy silt deposits which extended to the top of apparent bedrock at about 5.5 to 8.5 deep. These intermediate soil types exhibited SPT resistance values ranging from 25 blows per foot (bpf) to as high as 100 blows for 3 inches, with the higher blowcounts likely being obtained in apparent weathered shale/siltstone bedrock.

Hard drilling on apparent shale/siltstone bedrock with varying amounts of weathering was encountered in the TSC borings at depths ranging from about 3 to 8.5 feet below existing grade (approximate Elevations 477 to 483). The shale/siltstone materials were drilled using augers and sampled using split-barrel (split-spoon) methods to a depth of 10 feet below existing grade. They exhibited Standard Penetration Test (SPT) resistance values ranging from 93 blows for 7 inches to as high as 100 blows for 2 inches. Moisture contents of the split-spoon samples ranged from 5 to 12 percent.

Hard drilling on apparent bedrock (described as Very Dense Weathered Shale sometimes with limestone "stringers" or "caprock") was also encountered in the MTS borings at depths ranging from 2 to 7 feet below existing grade (approximate Elevations 480 - 486). The borings were extended into the bedrock materials to depths typically ranging from 3 to 8.5 feet below existing grade, up to 20 feet deep in Borings B-19 and B-20.

Free groundwater was initially revealed at depths of 5.5 to 10.5 feet below existing grade in Borings B2-2, B2-4 and B2-11. Upon completion of drilling and sampling operations, B2-2 was observed in a dry condition, the water level in B2-4 had remained at 10.5 feet deep and the water level in B2-11 had dropped to a depth of 14 feet.

Analysis and Preliminary Recommendations

Bearing Table

As previously discussed, it is assumed that the development will include the eventual construction of 1 to 2-story, slab-on-grade light industrial structures. No building plans and grading information were available at the time this report was prepared, e.g. top of finished floor (FF) elevations for the various buildings have not been set.

Industrial buildings are often supported on footing foundations designed for a net allowable bearing pressure of 3000 pounds per square foot (psf). Based on the results of the borings, this is considered to be achievable at this site. However, some remedial measures need to be taken at some locations due to the presence of relatively soft/high moisture content silty clay and organic clay deposits. Summarized in the following table are the anticipated minimum required depths of soil stripping/ undercutting to construct a “structural” building pad or find soils/materials suitable for supporting a net allowable bearing pressure of 3000 psf. Ground surface elevations and depths of surficial topsoil are

also shown. Added notes relate to undercut depths for mass-grading/building pad construction (U) and marginal bearing soils for fill placement and/or foundation support (M). The recommended bearing stress of 3000 psf is typical and generally satisfactory for the construction of industrial buildings in this area. Note that these recommendations are preliminary and need to be revisited once project specifics become available. Additional exploration will likely be recommended at that time.

Boring Number	Ground Surface Elevation	Topsoil Depth (Feet)	3000 PSF Design Bearing - Building Pad Undercut	
			Depth (Feet)*	Elevation*
B3-1	488.5	3.0	3.0	485.5
B3-2	485.3	1.2	5.5 U	479.5
B3-3	485.6	3.0	3.0	482.5
B3-4	485.7	1.1	4.5 U	481.0
B3-5	488.4	1.0	1.0	487.5
SB-1	485.6	1.3	1.5 M	484.0
SB-2	485.8	1.0	1.0	484.5
SB-3	488.2	1.0	1.0	487.0
SB-4	488.6	1.0	1.0	487.5
SB-5	488.4	0.7	1.0	487.5
SB-6	486.1	1.1	1.5 M	484.5
SB-7	487.2	1.0	1.0	486.0
SB-8	488.3	1.1	1.5 M	486.5
B-16	N/A	1.3	1.5	--
B-17	N/A	1.3	1.5	--
B-18	N/A	1.3	1.5 M	--
B-19	N/A	2.5	2.5	--
B-20	N/A	2.5	2.5	--

* Depth/elevation to 3000 psf bearing rounded to lowest 0.5 foot.

N/A The ground surface elevation at the boring location was not provided.

M Marginal bearing soils for fill placement and/or foundation support.

U Undercut depth to remove compressive soils as part of building pad construction.

Mass-Grading/Building Pad Construction

It is recommended that building pad (and pavement) areas be cleared of any vegetation, topsoil and root zone materials prior to mass-grading. Building pad construction will also require that the organic silty clay and high moisture content clay soils be removed and replaced to the approximate depths shown in the above table for Borings B3-2 and B3-4, i.e. to depths on the order of 5 feet below existing grade.

Undercutting of the above described compressive soils will require that the building pads be enlarged to permit the horizontal distribution of footing loads. It is recommended that the base of the undercut, or zone of stripping where only surficial topsoil/root zone materials are to be removed, extend a minimum of 5 feet outside the outer edge of the structures plus 0.5 feet for every foot of new fill to be placed.

The building pad and pavement areas should otherwise be proof-rolled using a loaded dump truck or other approved pieces of heavy rubber-tired construction equipment. This would aid in detecting the presence of additional unsuitable soil types. All soft or unstable materials determined by proof-rolling should be removed and replaced or appropriately remediated.

Marginal subgrade stability represented by native clay soils with unconfined compressive strengths between 0.7 and 1.5 tsf was encountered directly below the topsoil layer in MTS Borings SB-1, SB-6, SB-8 and SB-18. These soils may need to be undercut and replace with 1 to 2 feet of coarse granular materials. Lime or cement stabilization can achieve similar results and also have the advantage of allowing work to proceed under adverse weather conditions. In any event, the need for subgrade reworking, additional undercutting or lime/cement stabilization should be evaluated on the basis of proof-rolling.

The traffic of heavy construction equipment frequently causes clayey silt and sandy silt deposits encountered at shallow depths in some of the borings to experience a short-term decrease in stability. The associated soft and spongy condition of exposed soils is commonly referred to as "pumping" in this area. It is recommended that heavy construction equipment be detoured around any areas where pumping conditions are found to be developing. Depending upon grading requirements and specific site conditions, solutions to a persistent pumping problem may include removal or the unstable soils, placement of a bridging lift of granular backfill, use of geotextile stabilization fabric or geogrid products in conjunction with the granular fill or a combination thereof. Other developments in the area have used lower-impact “cat and pan” equipment in lieu of “scrapers” to alleviate instability and pumping.

New fill should otherwise consist of inorganic silty clays of low to medium plasticity or approved granular materials. It is recommended that compaction of building pad and pavement areas be to a minimum of 95 and 90 percent of maximum dry density, respectively, as determined by the Modified Proctor test (ASTM D1557). The upper 2 feet of pavement subgrade should also be compacted to the 95 percent criterion, to create a more stable base for final proof-rolling prior to paving. The fill should be placed in approximate 10-inch lifts loose measure for cohesive soils and up to 12 inches for granular materials, each lift to be compacted to the specified density prior to the placement of additional fill.

Moisture control is important in the compaction of most soil types, and it is recommended that the water content of new fill be within about 1 percentage point on the low side and 3 percentage points on the high side of optimum moisture as established by its laboratory compaction curve. If the soil is compacted on the dry side, it will have an apparent stability which will be lost if it later becomes saturated. If the soil is too wet, the Contractor will not be able to achieve proper compaction.

Building Foundations

New engineered fill placed as part of building pad construction will be suitable for 3000 psf bearing in connection with foundation design and construction. This is provided that the new fill bears upon the native 3000 psf bearing materials. The underlying stiff to hard native clay and medium dense to very dense silt soils as well as shale/siltstone bedrock are also suitable for 3000 psf bearing.

It is recommended that all continuous wall footings be made at least 24 inches wide and isolated foundations at least 3.0 feet square, regardless of calculated dimensions. For frost considerations, all exterior footings should be constructed at least 3.5 feet below outside finished grade and 4.0 feet for foundations located outside of heated building limits. Interior footings may be constructed at higher elevations as long as they are protected against frost heave in the event of winter construction.

Ground Supported Slabs

A modulus of subgrade reaction of 150 pounds per cubic inch (pci) may be used for concrete floor slab design. However, for sustained loads such as racking system leg loads, the subgrade modulus should be reduced to 100 pci or lower, dependent on the loading and loaded footprint. Use of these values assumes that new fill and uppermost soils are compacted to 95 percent Modified Proctor density as part of building pad preparation.

In connection with slab-on-grade construction, it is also recommended that a minimum 4 to 6-inch thick granular base be utilized for load distribution and as leveling course and capillary break. Base course materials should consist of gravel having a maximum size of about one inch and containing less than about 10 percent fines (passing No. 200 sieve). Typical base course materials include IDOT gradation CA-6 (well-graded sand and gravel mixture with fines) and CA-7 (¼ to ¾ inch chips). The CA-7 material is considered to be free-draining, providing a superior capillary break. The CA-6 material should be compacted to 95 percent Modified Proctor density, CA-7 rolled to a dense and stable state.

The concrete slabs should be designed and jointed to minimize the effects of any slab movement and related cracking. In this regard, slab-on-grade construction and jointing should be in accordance with ACI 360-10 (Guide to Design of Slabs-on-Ground).

Pavement Design and Construction

Pavement subgrade preparation may be in general accordance with previous recommendations for mass-grading, i.e. strip topsoil and root zone materials and proof-roll. It is anticipated that existing subgrade will in some areas have to be reduced in moisture content and recompacted, to be based on proof-rolling. Compaction to at least 90 percent Modified Proctor density is recommended in pavement

areas. However, at least the upper 2 feet of pavement subgrade is usually compacted to 95 percent Modified Proctor density to create a more stable base for final proof-rolling and paving. If paving construction is performed when drying of surficial soils cannot be accomplished, lime or cement modification or removal of unstable subgrade and replacement with 1 to 2 feet of coarse granular materials may be required in some areas.

Pumping of silt and loamy soil types, as discussed under mass-grading, is typically more of a problem in pavement areas than for building pads. This condition is likely to require undercuts and the use of geotextile stabilization fabric or geogrid products and coarse aggregate backfill for a portion of pavement areas. Subgrade stability will also be affected by weather conditions at the time of paving.

A nominal California Bearing Ratio (CBR) value of 3.0 may be utilized for the design of asphalt pavements. A subgrade modulus of 150 pci may be used for the design of P.C. concrete pavements. Use of these values assume that any soft or unstable areas will be remediated, i.e. subgrade stabilized until passing a proof-roll.

Base course materials should conform to IDOT gradation specification CA-6 and be compacted to 95 percent Modified Proctor density or 100 percent of the Standard Proctor (ASTM D 698) maximum density value. Bituminous materials should conform to an approved current IDOT Superpave mix design (N30 or N50 typical for pavements with light traffic and N50 or N70 for pavements with heavy truck traffic), as well as Standard Specifications for Road and Bridge Construction, Sections 406 and 1032. They should be compacted to between 93 and 97 percent of their theoretical maximum density, as determined by the supplier.

Portland Cement Concrete (PCC) or heavy duty asphaltic concrete is recommended for pavements with heavy truck traffic and high traffic load areas such as garbage truck dumpster loading areas and aprons approaching parking areas. Standard Specifications for Road and Bridge Construction, Sections 353 and 420, should be followed.

Stormwater Management Area

It is understood that the expansion of an existing detention pond may be considered along the south end of the site. MTS Borings B-16 thru B-20 were drilled in the area of the potential detention basin expansion, being extended to 6 to 20 feet below existing grade. Bedrock (described as Very Dense Weathered Shale and Shale with Limestone Stringers) was first encountered at a about 3 to 7 feet below existing grade in these borings, with stiff clay soils predominating above rock. These materials are expected to be stable on the 3H:1V or gentler side slopes typically used for detention basins in this area. In any event, side slopes steeper than 3H:1V are not recommended considering the soil and rock types encountered by the borings.

Consideration could be given to partially removing the highly-weathered shale/siltstone bedrock from the side slopes and possibly the bottom of the basin excavations to help prevent side slope erosion and sloughing. They should be replaced with cohesive materials for a distance of at least 1 to 2 feet perpendicular to the cut. "Capping" of highly-weathered shale/siltstone bedrock will protect the side slopes from erosion.

Replacement materials should consist of clay soil types of medium to high plasticity, ideally containing less than 35 percent sand and gravel size particles. The liner materials should be placed in approximate 10 inch lifts loose measure and compacted to at least 90 percent of maximum dry density as determined by the Modified Proctor test (ASTM D 1557). At the time of placement and compaction the clay fill should also be on the wet side of optimum moisture content, as determined by the laboratory compaction curve. It may also be possible that the topsoil at the site could be used for this purpose. However, it would be necessary to obtain bulk topsoil samples to test them for suitability, i.e. clay content.

Rock Excavation

As previously discussed, hard drilling and/or high sampler blow counts were encountered at depths ranging from about 2 to 8 feet below existing grade at the boring locations. This was due to the presence of shale/siltstone bedrock. Depending on final grades, these materials may be encountered during excavation for building foundations, underground utilities and detention basins. It may not be possible to excavate these materials using normal backhoe equipment. Special excavation procedures may be required for foundation, underground utility and detention basin excavations, possibly to include rock excavation methods.

In some cases our drilling equipment can penetrate materials which are difficult to dig with conventional excavation equipment. Conversely, sometimes our equipment may experience hard drilling or refusal which can be excavated by backhoe-type equipment. In this regard, it would be advisable to excavate test pits prior to construction to assess the rippability of the materials at the site, i.e. the ease or difficulty of excavation. Specialized rock excavation methods may be required where relatively sound rock is encountered.

Groundwater Management

Major groundwater problems are not anticipated taking into consideration the fine-grained nature (i.e. low permeability) of subsurface soils as well as the fact that most of the recent borings didn't encountered free water during drilling and sampling operations. However, shallow groundwater levels are indicated on the logs for some of the previous MTS borings. The Contractor should therefore be prepared to implement dewatering/unwatering procedures during construction, as a minimum to include pumping from strategically placed sumps.

Closure

This preliminary report has been prepared using widely-spaced borings and without the benefit of building, site and grading plans. It is recommended that additional borings be performed when building and site plans are available to fill in gaps in information, especially in the areas where poor (compressible) soils were encountered.

The analysis and preliminary recommendations submitted in this report are based upon the data obtained from the five (5) soil borings drilled by TSC and the thirteen (13) previous borings performed by MTS at the locations indicated on the Boring Location Plan. This report does not reflect any

variations which may occur between these borings or elsewhere on the site, the nature and extent of which may not become evident until during the course of construction. If variations are then identified, recommendations contained in this report should be re-evaluated after performing on-site observations.

It has been a pleasure to assist you with this work. Please call if there are any questions or if we may be of further service.

Respectfully submitted,

TESTING SERVICE CORPORATION

Alfredo J. Bermudez
Senior Geotechnical Engineer
Registered Professional Engineer
Illinois No. 062-046608

AJB:SRK:ab
Enc.

Steven R. Koester, P.E.
Vice President

APPENDIX

GENERAL CONDITIONS

UNIFIED CLASSIFICATION CHART

LEGEND FOR BORING LOGS

BORING LOGS

BORING LOCATION PLAN

GENERAL CONDITIONS

Geotechnical and Construction Services

TESTING SERVICE CORPORATION

1. PARTIES AND SCOPE OF WORK: If Client is ordering the services on behalf of another, Client represents and warrants that Client is the duly authorized agent of said party for the purpose of ordering and directing said services, and in such case the term "Client" shall also include the principal for whom the services are being performed. Prices quoted and charged by TSC for its services are predicated on the conditions and the allocations of risks and obligations expressed in these General Conditions. Unless otherwise stated in writing, Client assumes sole responsibility for determining whether the quantity and the nature of the services ordered by Client are adequate and sufficient for Client's intended purpose. Unless otherwise expressly assumed in writing, TSC's services are provided exclusively for client. TSC shall have no duty or obligation other than those duties and obligations expressly set forth in this Agreement. TSC shall have no duty to any third party. Client shall communicate these General Conditions to each and every party to whom the Client transmits any report prepared by TSC. Ordering services from TSC shall constitute acceptance of TSC's proposal and these General Conditions.

2. SCHEDULING OF SERVICES: The services set forth in this Agreement will be accomplished in a timely and workmanlike manner. If TSC is required to delay any part of its services to accommodate the requests or requirements of Client, regulatory agencies, or third parties, or due to any cause beyond its reasonable control, Client agrees to pay such additional charges, if any, as may be applicable.

3. ACCESS TO SITE: TSC shall take reasonable measures and precautions to minimize damage to the site and any improvements located thereon as a result of its services or the use of its equipment; however, TSC has not included in its fee the cost of restoration of damage which may occur. If Client desires or requires TSC to restore the site to its former condition, TSC will, upon written request, perform such additional work as is necessary to do so and Client agrees to pay to TSC the cost thereof plus TSC's normal markup for overhead and profit.

4. CLIENT'S DUTY TO NOTIFY ENGINEER: Client represents and warrants that Client has advised TSC of any known or suspected hazardous materials, utility lines and underground structures at any site at which TSC is to perform services under this Agreement. Unless otherwise agreed in writing, TSC's responsibility with respect to underground utility locations is to contact the Illinois Joint Utility Locating Information for Excavators for the location of public, but not private, utilities.

5. DISCOVERY OF POLLUTANTS: TSC's services shall not include investigation for hazardous materials as defined by the Resource Conservation Recovery Act, 42 U.S.C.§ 6901, et, seq., as amended ("RCRA") or by any state or Federal statute or regulation. In the event that hazardous materials are discovered and identified by TSC, TSC's sole duty shall be to notify Client.

6. MONITORING: If this Agreement includes testing construction materials or observing any aspect of construction of improvements, Client's construction personnel will verify that the pad is properly located and sized to meet Client's projected building loads. Client shall cause all tests and inspections of the site, materials and work to be timely and properly performed in accordance with the plans, specifications, contract documents, and TSC's recommendations. No claims for loss, damage or injury shall be brought against TSC unless all tests and inspections have been so performed and unless TSC's recommendations have been followed.

TSC's services shall not include determining or implementing the means, methods, techniques or procedures of work done by the contractor(s) being monitored or whose work is being tested. TSC's services shall not include the authority to accept or reject work or to in any manner supervise the work of any contractor. TSC's services or failure to perform same shall not in any way operate or excuse any contractor from the performance of its work in accordance with its contract. "Contractor" as used herein shall include subcontractors, suppliers, architects, engineers and construction managers.

Information obtained from borings, observations and analyses of sample materials shall be reported in formats considered appropriate by TSC unless directed otherwise by Client. Such information is considered evidence, but any inference or conclusion based thereon is, necessarily, an opinion also based on engineering judgment and shall not be construed as a representation of fact. Subsurface conditions may not be uniform throughout an entire site and ground water levels may fluctuate due to climatic and other variations. Construction materials may vary from the samples taken. Unless otherwise agreed in writing, the procedures employed by TSC are not designed to detect intentional concealment or misrepresentation of facts by others.

7. DOCUMENTS AND SAMPLES: Client is granted an exclusive license to use findings and reports prepared and issued by TSC and any sub-consultants pursuant to this Agreement for the purpose set forth in TSC's proposal provided that TSC has received payment in full for its services. TSC and, if applicable, its sub-consultant, retain all copyright and ownership interests in the reports, boring logs, maps, field data, field notes, laboratory test data and similar documents, and the ownership and freedom to use all data generated by it for any purpose. Unless otherwise agreed in writing, test specimens or samples will be disposed immediately upon completion of the test. All drilling samples or specimens will be disposed sixty (60) days after submission of TSC's report.

8. TERMINATION: TSC's obligation to provide services may be terminated by either party upon (7) seven days prior written notice. In the event of termination of TSC's services, TSC shall be compensated by Client for all services performed up to and including the termination date, including reimbursable expenses. The terms and conditions of these General Conditions shall survive the termination of TSC's obligation to provide services.

9. PAYMENT: Client shall be invoiced periodically for services performed. Client agrees to pay each invoice within thirty (30) days of its receipt. Client further agrees to pay interest on all amounts invoiced and not paid or objected to in writing for valid cause within sixty (60) days at the rate of twelve (12%) per annum (or the maximum interest rate permitted by applicable law, whichever is the lesser) until paid and TSC's costs of collection of such accounts, including court costs and reasonable attorney's fees.

10. WARRANTY: TSC's professional services will be performed, its findings obtained and its reports prepared in accordance with these General Conditions and with generally accepted principles and practices. In performing its professional services, TSC will use that degree of care and skill ordinarily exercised under similar circumstances by members of its profession. In performing physical work in pursuit of its professional services, TSC will use that degree of care and skill ordinarily used under similar circumstances. This warranty is in lieu of all other warranties or representations, either express or implied. Statements made in TSC reports are opinions based upon engineering judgment and are not to be construed as representations of fact.

Should TSC or any of its employees be found to have been negligent in performing professional services or to have made and breached any express or implied warranty, representation or contract, Client, all parties claiming through Client and all parties claiming to have in any way relied upon TSC's services or work agree that the maximum aggregate amount of damages for which TSC, its officers, employees and agents shall be liable is limited to $50,000 or the total amount of the fee paid to TSC for its services performed with respect to the project, whichever amount is greater.

In the event Client is unwilling or unable to limit the damages for which TSC may be liable in accordance with the provisions set forth in the preceding paragraph, upon written request of Client received within five days of Client's acceptance of TSC's proposal together with payment of an additional fee in the amount of 5% of TSC's estimated cost for its services (to be adjusted to 5% of the amount actually billed by TSC for its services on the project at time of completion), the limit on damages shall be increased to $500,000 or the amount of TSC's fee, whichever is the greater. This charge is not to be construed as being a charge for insurance of any type, but is increased consideration for the exposure to an award of greater damages.

11. INDEMNITY: Subject to the provisions set forth herein, TSC and Client hereby agree to indemnify and hold harmless each other and their respective shareholders, directors, officers, partners, employees, agents, subsidiaries and division (and each of their heirs, successors, and assigns) from any and all claims, demands, liabilities, suits, causes of action, judgments, costs and expenses, including reasonable attorneys' fees, arising, or allegedly arising, from personal injury, including death, property damage, including loss of use thereof, due in any manner to the negligence of either of them or their agents or employees or independent contractors. In the event both TSC and Client are found to be negligent or at fault, then any liability shall be apportioned between them pursuant to their pro rata share of negligence or fault. TSC and Client further agree that their liability to any third party shall, to the extent permitted by law, be several and not joint. The liability of TSC under this provision shall not exceed the policy limits of insurance carried by TSC. Neither TSC nor Client shall be bound under this indemnity agreement to liability determined in a proceeding in which it did not participate represented by its own independent counsel. The indemnities provided hereunder shall not terminate upon the termination or expiration of this Agreement, but may be modified to the extent of any waiver of subrogation agreed to by TSC and paid for by Client.

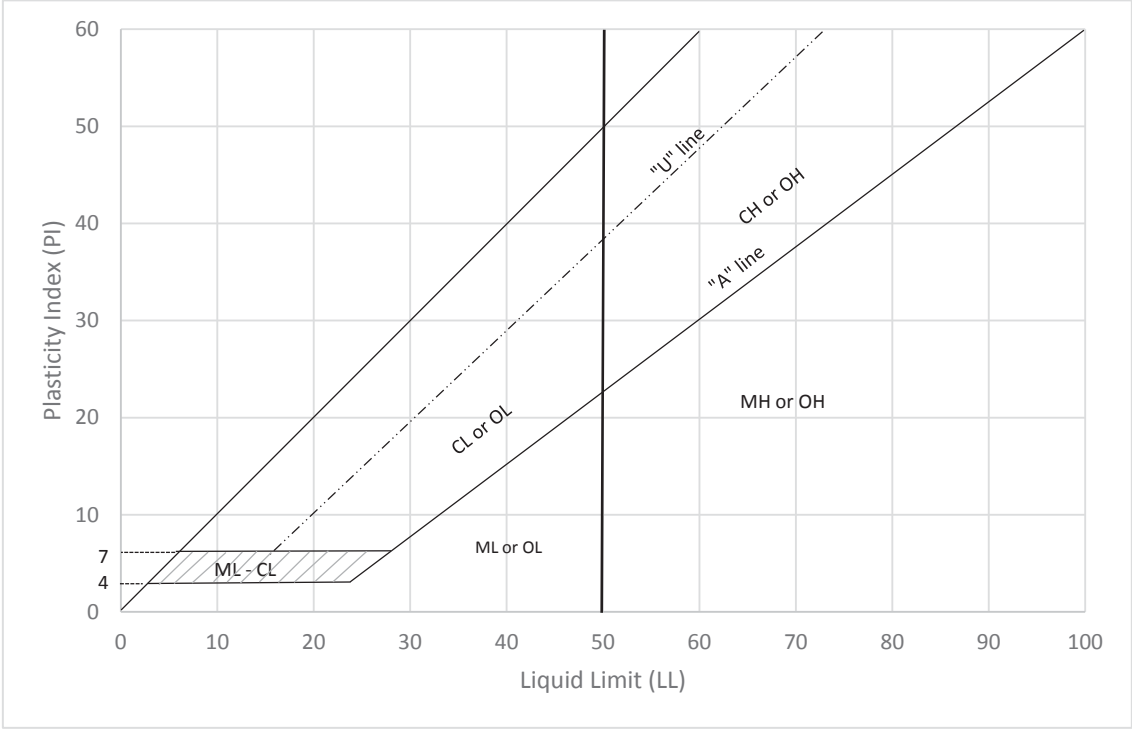
12. SUBPOENAS: TSC's employees shall not be retained as expert witnesses except by separate, written agreement. Client agrees to pay TSC pursuant to TSC's then current fee schedule for any TSC employee(s) subpoenaed by any party as an occurrence witness as a result of TSC's services.

13. OTHER AGREEMENTS: TSC shall not be bound by any provision or agreement (i) requiring or providing for arbitration of disputes or controversies arising out of this Agreement or its performance, (ii) wherein TSC waives any rights to a mechanics lien or surety bond claim; (iii) that conditions TSC's right to receive payment for its services upon payment to Client by any third party or (iv) that requires TSC to indemnify any party beyond its own negligence These General Conditions are notice, where required, that TSC shall file a lien whenever necessary to collect past due amounts. This Agreement contains the entire understanding between the parties. Unless expressly accepted by TSC in writing prior to delivery of TSC's services, Client shall not add any conditions or impose conditions which are in conflict with those contained herein, and no such additional or conflicting terms shall be binding upon TSC. The unenforceability or invalidity of any provision or provisions shall not render any other provision or provisions unenforceable or invalid. This Agreement shall be construed and enforced in accordance with the laws of the State of Illinois. In the event of a dispute arising out of or relating to the performance of this Agreement, the breach thereof or TSC's services, the parties agree to try in good faith to settle the dispute by mediation under the Construction Industry Mediation Rules of the American Arbitration Association as a condition precedent to filing any demand for arbitration, or any petition or complaint with any court. Paragraph headings are for convenience only and shall not be construed as limiting the meaning of the provisions contained in these General Conditions.

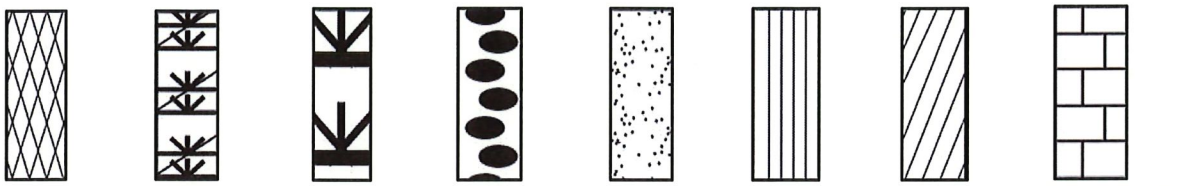
Testing Service Corporation
Unified Classification Chart

CRITERIA FOR ASSIGNING GROUP SYMBOLS AND GROUP NAMES USING LABORATORY TEST [a]				SOIL CLASSIFICATION	
				Group Symbol	GROUP NAME [b]
COARSE - GRAINED SOILS more than 50% retained on No. 200 sieve	GRAVELS More than 50% of coarse fraction retained on No. 4 sieve	CLEAN GRAVELS less than 5% fines [c]	$C_u \geq 4$ and $1 \leq C_C \leq 3$ [e]	GW	Well-graded gravel [f]
			$C_u < 4$ and/or $1 > C_C > 3$ [e]	GP	Poorly-graded gravel [f]
		GRAVELS WITH FINES more than 12% fines [c]	Fines classify as ML or MH	GM	Silty gravel [f, g, h]
			Fines classify as CL or CH	GC	Clayey gravel [f, g, h]
	SANDS 50% or more of coarse fraction passes No. 4 sieve	CLEAN SANDS less than 5% fines [d]	$C_u \geq 6$ and $1 \leq C_C \leq 3$ [e]	SW	Well-graded sand [i]
			$C_u < 6$ and/or $1 > C_C > 3$ [e]	SP	Poorly-graded sand [i]
		SANDS WITH FINES more than 12% fines [d]	Fines classify as ML or MH	SM	Silty sand [g, h, f]
			Fines classify as CL or CH	SC	Clayey sand [g, h, f]
FINE - GRAINED SOILS 50% or more passed the No. 200 sieve	SILTS & CLAYS Liquid limit less than 50%	Inorganic	PI > 7 or plots on or above "A" line j	CL	Lean clay [k, l, m]
			PI < 4 or plots below "A" line j	ML	Silt [k, l, m]
		Organic	$\frac{\text{Liquid limit – oven dried}}{\text{Liquid limit – not dried}} < 0.75$	OL	Organic clay [k, l, m, n] Organic silt [k, l, m, o]
	SILTS & CLAYS Liquid limit 50% or more	Inorganic	PI plots on or above "A" line	CH	Fat clay [k, l, m]
			PI plots below "A" line	MH	Elastic silt [k, l, m]
		Organic	$\frac{\text{Liquid limit – oven dried}}{\text{Liquid limit – not dried}} < 0.75$	OH	Organic clay [k, l, m, p] Organic silt [k, l, m, q]
Highly organic soils		Primarily organic matter, dark in color, and organic odor		PT	Peat

a. Based on the material passing the 3-inch (75-mm) sieve.
b. If field sample contained cobbles and/or boulders, add "with cobbles and/or boulders" to group name
c. Gravels with 5 to 12% fines required dual symbols
GW-GM well graded gravel with silt
GW-GC well graded gravel with clay
GP-GM poorly graded gravel with silt
GP-GC poorly graded gravel with clay
d. Sands with 5 to 12% fines require dual symbols
SW-SM well graded sand with silt
SW-SC well graded sand with clay
SP-SM poorly graded sand with silt
SP-SC poorly graded sand with clay
e. $C_u = D_{60}/D_{10}$ $C_C = \frac{(D_{30})^2}{D_{10} \times D_{60}}$
f. If soils contains ≥ 15% sand, add "with sand" to group name.
g. If fines classify as CL-ML, use dual symbol GC-GM, SC-SM
h. If fines are organic, add "with organic fines" to group name
i. If soils contains ≥ 15% gravel, add "with gravel" to group name
j. If Atterberg Limits plot in hatched area, soil is a CL – ML, silty clay
k. If soils contains 15 to 29% plus No. 200, add "with sand" or "with gravel" whichever is predominant
l. If soil contains ≥ 30% plus No. 200, predominantly sand, add "sandy" to group name.
m. If soils contains ≥ 30% plus No. 200, predominantly gravel, add "gravelly" to group name
n. PI ≥ 4 and plots on or above "A" line
o. PI ≥ 4 and plots below "A" line
p. PI plots on or above "A" line
q. PI plots below "A" line



LEGEND FOR BORING LOGS



FILL | TOPSOIL | PEAT | GRAVEL | SAND | SILT | CLAY | LIMESTONE/ DOLOMITE

SAMPLE TYPE

SS	=	Split-Spoon
ST	=	Thin-Walled Tube
A	=	Auger
MC	=	Macro-Core (Geoprobe)
RC	=	Rock Core

WATER LEVEL OBSERVATIONS

▼	While Drilling
▽	End of Boring
▼	24 Hours

FIELD AND LABORATORY TEST DATA

N	=	Standard Penetration Resistance in Blows per Foot (bpf)
WC	=	In-Situ Water Content (%)
Qu	=	Unconfined Compressive Strength in Tons per Square Foot (tsf)
Qp	=	Pocket Penetrometer Reading: Maximum Value = 4.5 tsf
γ_{dry}	=	Dry Unit Weight in Pounds per Cubic Foot (pcf)

SOIL DESCRIPTIONS:

MATERIAL	PARTICLE SIZE RANGE
BOULDER	Over 12 inches
COBBLE	12 inches to 3 inches
Large GRAVEL	3 inches to ¾ inch
Small GRAVEL	¾ inch to No. 4 Sieve
Coarse SAND	No. 4 Sieve to No. 10 Sieve
Medium SAND	No. 10 Sieve to No. 40 Sieve
Fine SAND	No. 40 Sieve to No. 200 Sieve
SILT and CLAY	Passing No. 200 Sieve

COHESIVE SOILS

CONSISTENCY	Qu (tsf)
Very Soft	Less than 0.25
Soft	0.25 to 0.5
Medium Stiff	0.5 to 1.0
Stiff	1.0 to 2.0
Very Stiff	2.0 to 4.0
Hard	4.0 and over

COHESIONLESS SOILS

RELATIVE DENSITY	N (bpf)
Very Loose	0 – 3
Loose	4 – 9
Medium Dense	10 – 29
Dense	30 – 49
Very Dense	50 and over

MODIFYING TERM	PERCENT BY WEIGHT
Trace	1 – 10
Little	10 – 20
Some	20 – 35

PROJECT **Ottawa Industrial Park, Unit 3, US Route 6, East of Route 71, Ottawa, IL**

CLIENT **City of Ottawa, 301 West Madison Street, Ottawa, IL**

STARTED **10/29/2025** FINISHED **10/29/2025** DRILL RIG NO. **256** JOB **L-99,961**

BORING **B3-1**

ELEVATIONS

GROUND SURFACE **488.5**

END OF BORING **478.5**

WATER LEVEL OBSERVATIONS

WHILE DRILLING **Dry**

AT END OF BORING **Dry**

Depth (ft.)	Lithology	Length	Recovery	SAMPLE NO.	SAMPLE TYPE	N	WC (%)	Qp (tsf)	Qu (tsf)	γ DRY (pcf)	DEPTH (ft)	ELEV.	SOIL DESCRIPTIONS
0				1	SS	8	30.7	3.5					Dark brown to black clayey TOPSOIL, very moist (OL) [Possible Partial Fill] LOI = 6.4%
											3.0	485.5	
				2A	SS	9-15-50/1"	10.0						Medium dense brown trace gray sandy SILT, trace clay, moist (ML)
				2B			9.1				4.5	484.0	
5													Very dense brown sandy SILT, moist (ML)
				3	SS	100/2"	9.0				5.5	483.0	
													Gray weathered SHALE/SILTSTONE, damp [Hard Drilling]
				4	SS	100/2"	7.0						
10													End of Boring at 10.0'
													CME Automatic Hammer used for SPTs
15													
20													

Division lines between deposits represent approximate boundaries between soil types; in-situ, the transition may be gradual.

PROJECT **Ottawa Industrial Park, Unit 3, US Route 6, East of Route 71, Ottawa, IL**

CLIENT **City of Ottawa, 301 West Madison Street, Ottawa, IL**

STARTED **10/29/2025** FINISHED **10/29/2025** DRILL RIG NO. **256** JOB **L-99,961**

BORING **B3-2**

ELEVATIONS

GROUND SURFACE **485.3**

END OF BORING **475.3**

WATER LEVEL OBSERVATIONS

WHILE DRILLING **5.5'**

AT END OF BORING **8.0'**

Depth (ft.)	Lithology	Length	Recovery	SAMPLE NO.	SAMPLE TYPE	N	WC (%)	Qp (tsf)	Qu (tsf)	γ DRY (pcf)	DEPTH (ft)	ELEV.	SOIL DESCRIPTIONS
0													Black clayey TOPSOIL (OL)
											1.2	484.1	
				1	SS	5	47.7	2.0					Stiff to very stiff dark brown silty CLAY, trace sand, little Organic, very moist (CH)
											3.0	482.3	
				2	SS	2	64.4	0.5					Soft dark brown ORGANIC SILTY CLAY, very moist (OH)
5											5.5	479.8	
				3	SS	54	11.7						Very dense gray clayey SILT, little sand, moist (ML)
				4	SS	35-50/2"	12.3				8.5	476.8	Gray weathered SHALE/SILTSTONE, moist [Hard Drilling]
10													End of Boring at 10.0'
													CME Automatic Hammer used for SPTs
15													
20													

Division lines between deposits represent approximate boundaries between soil types; in-situ, the transition may be gradual.

PROJECT **Ottawa Industrial Park, Unit 3, US Route 6, East of Route 71, Ottawa, IL**

CLIENT **City of Ottawa, 301 West Madison Street, Ottawa, IL**

STARTED **10/29/2025** FINISHED **10/29/2025** DRILL RIG NO. **256** JOB **L-99,961**

BORING **B3-3**

ELEVATIONS

GROUND SURFACE **485.6**

END OF BORING **475.6**

WATER LEVEL OBSERVATIONS

WHILE DRILLING **Dry**

AT END OF BORING **Dry**

Depth (ft.)	Lithology	Length	Recovery	SAMPLE NO.	SAMPLE TYPE	N	WC (%)	Qp (tsf)	Qu (tsf)	γ DRY (pcf)	DEPTH (ft)	ELEV.	SOIL DESCRIPTIONS
0				1	SS	6	28.0	3.25					Dark brown to black clayey TOPSOIL, moist (OL) [Possible Partial Fill]
											3.0	482.6	
				2	SS	75-50/1"	8.4						
5				3	SS	53-50/1"	7.3						Gray weathered SHALE/SILTSTONE, damp [Hard Drilling]
				4	SS	100/3"	7.1						
10													End of Boring at 10.0' CME Automatic Hammer used for SPTs
15													
20													

Division lines between deposits represent approximate boundaries between soil types; in-situ, the transition may be gradual.

PROJECT **Ottawa Industrial Park, Unit 3, US Route 6, East of Route 71, Ottawa, IL**

CLIENT **City of Ottawa, 301 West Madison Street, Ottawa, IL**

STARTED **10/29/2025** FINISHED **10/29/2025** DRILL RIG NO. **256** JOB **L-99,961**

BORING **B3-4**

ELEVATIONS

GROUND SURFACE **485.7**

END OF BORING **475.7**

WATER LEVEL OBSERVATIONS

WHILE DRILLING **Dry**

AT END OF BORING **Dry**

Depth (ft.)	Lithology	Length	Recovery	SAMPLE NO.	SAMPLE TYPE	N	WC (%)	Qp (tsf)	Qu (tsf)	γ DRY (pcf)	DEPTH (ft)	ELEV.	SOIL DESCRIPTIONS
0													Black clayey TOPSOIL (OL)
											1.1	484.6	
				1	SS	6	37.2	2.25					Very stiff dark brownish-gray silty CLAY, trace sand, little Organic, trace roots and snail shell fragments, very moist (CH)
											3.0	482.7	
				2A	SS	3-3-50/3"	39.1	0.75					Medium stiff brownish-gray silty CLAY, trace sand, trace organic, very moist (CL/CH)
				2B			7.6				4.5	481.2	
5				3	SS	36-43-50/1"	7.6						Light gray to gray weathered SHALE/SILTSTONE, damp [Hard Drilling]
				4	SS	100/3"	7.2						
10													End of Boring at 10.0'
													CME Automatic Hammer used for SPTs
15													
20													

Division lines between deposits represent approximate boundaries between soil types; in-situ, the transition may be gradual.

PROJECT **Ottawa Industrial Park, Unit 3, US Route 6, East of Route 71, Ottawa, IL**

CLIENT **City of Ottawa, 301 West Madison Street, Ottawa, IL**

STARTED **10/29/2025** FINISHED **10/29/2025** DRILL RIG NO. **256** JOB **L-99,961**

BORING **B3-5**

ELEVATIONS

GROUND SURFACE **488.4**

END OF BORING **478.4**

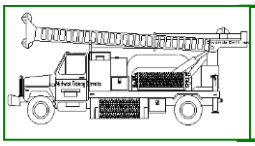
WATER LEVEL OBSERVATIONS

WHILE DRILLING **Dry**

AT END OF BORING **Dry**

Depth (ft.)	Lithology	Length	Recovery	SAMPLE NO.	SAMPLE TYPE	N	WC (%)	Qp (tsf)	Qu (tsf)	γ DRY (pcf)	DEPTH (ft)	ELEV.	SOIL DESCRIPTIONS
0													Black clayey TOPSOIL (OL)
											1.0	487.4	
				1	SS	7	24.6	4.25					Hard brown and gray silty CLAY, little sand, moist (CL/CH)
											3.0	485.4	
				2	SS	27	13.8						Medium dense brown trace gray clayey SILT, little sand, moist (ML)
5											5.5	482.9	
				3	SS	100/3"	13.1						Very dense brown and gray SILT, little sand, trace clay, moist (ML) [Possible highly weathered Shale/Siltstone]
											8.0	480.4	
				4	SS	100/2"	4.9						Gray weathered SHALE/SILTSTONE, damp [Hard Drilling]
10													End of Boring at 10.0'
													CME Automatic Hammer used for SPTs
15													
20													

Division lines between deposits represent approximate boundaries between soil types; in-situ, the transition may be gradual.



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BORING LOG

Sheet 1 of 1

Client: City of Ottawa
Project Name: Ottawa Industrial Park Unit 3
Project Site: Ottawa, Illinois

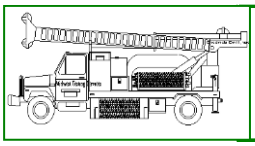
Boring No. SB-1
Surface Elev. 485.60
Auger Depth 5.5' Rotary Depth NA
Start Date 06/04/19 Finish Date 06/04/19

Location: Per Sketch

DRILLED BY: Randy Safranski

(DEPTH) ELEV.	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES: Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear	Moisture (%)	Dry Density (PCF)	REMARKS
485.60											
484.60	Black Silty Clay Topsoil 15"		1								First Water 484.3
483.60			2								
482.60	Medium Stiff Brownish Gray Silty Clay (CL)		3	1	SS	0.7	4	B	23		
481.60			4								
480.60	Very Dense Brown/Gray Weathered Shale		5	2	SS	---	100/4"	---	---		
479.60	Boring Terminated Auger Refusal		6								
478.60			7								
477.60			8								
476.60			9								
475.60			10								
474.60			11								
473.60			12								
472.60			13								
471.60			14								
470.60			15								
469.60			16								
468.60			17								
467.60			18								
466.60			19								
465.60			20								

Groundwater Data: Water after auger removal 16" below top of ground elevation.
Comments:



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BORING LOG

Sheet 1 of 1

Client: City of Ottawa
Project Name: Ottawa Industrial Park Unit 3
Project Site: Ottawa, Illinois

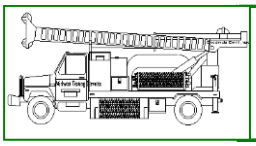
Boring No. SB-2
Surface Elev. 485.80
Auger Depth 5.0' Rotary Depth NA
Start Date 06/04/19 Finish Date 06/04/19

Location: Per Sketch

DRILLED BY: Randy Safranski

(DEPTH) ELEV.	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES: Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear	Moisture (%)	Dry Density (PCF)	REMARKS
485.80											
484.80	Black Silty Clay Topsoil 12"		1								First Water 485.5
483.80	Stiff Brownish Gray Silty Clay (CL)		2								
482.80			3	1	SS	1.5	10	B	21		
481.80			4								
480.80	Very Dense Brown/Gray Weathered Shale		5								
479.80	Boring Terminated Auger Refusal		6	2	SS	---	100/6"	---	---		
478.80			7								
477.80			8								
476.80			9								
475.80			10								
474.80			11								
473.80			12								
472.80			13								
471.80			14								
470.80			15								
469.80			16								
468.80			17								
467.80			18								
466.80			19								
465.80			20								

Groundwater Data: Water after auger removal 4" below top of ground elevation.
Comments:



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BORING LOG

Sheet 1 of 1

Client: City of Ottawa
Project Name: Ottawa Industrial Park Unit 3
Project Site: Ottawa, Illinois

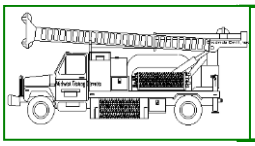
Boring No. SB-3
Surface Elev. 488.20
Auger Depth 4.0' Rotary Depth NA
Start Date 06/04/19 Finish Date 06/04/19

Location: Per Sketch

DRILLED BY: Randy Safranski

(DEPTH) ELEV.	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES: Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear	Moisture (%)	Dry Density (PCF)	REMARKS
488.20											
487.20	Black Silty Clay Topsoil 12"		1								First Water 487.2
486.20	Stiff Brownish Gray Silty Clay (CL)		2								
485.20			3	1	SS	1.8	12	B	20		
484.20	Very Dense Brown/Gray Weathered Shale		4								
483.20	Boring Terminated Auger Refusal		5								
482.20			6								
481.20			7								
480.20			8								
479.20			9								
478.20			10								
477.20			11								
476.20			12								
475.20			13								
474.20			14								
473.20			15								
472.20			16								
471.20			17								
470.20			18								
469.20			19								
468.20			20								

Groundwater Data: Water after auger removal 12" below top of ground elevation.
Comments:



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BORING LOG

Sheet 1 of 1

Client: City of Ottawa
Project Name: Ottawa Industrial Park Unit 3
Project Site: Ottawa, Illinois

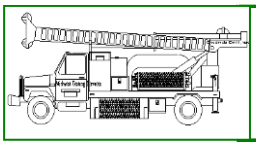
Boring No. SB-4
Surface Elev. 488.60
Auger Depth 3.0' Rotary Depth NA
Start Date 06/04/19 Finish Date 06/04/19

Location: Per Sketch

DRILLED BY: Randy Safranski

(DEPTH) ELEV.	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES: Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear	Moisture (%)	Dry Density (PCF)	REMARKS
488.60											
487.60	Black Silty Clay Topsoil 12"		1								
	Stiff Brownish Gray Silty Clay (CL)										
486.60			2								First Water $486.0
	Very Dense Brown/Gray Weathered Shale										
485.60	Boring Terminated Auger Refusal		3	1	SS	>4	30	B	20		
484.60			4								
483.60			5								
482.60			6								
481.60			7								
480.60			8								
479.60			9								
478.60			10								
477.60			11								
476.60			12								
475.60			13								
474.60			14								
473.60			15								
472.60			16								
471.60			17								
470.60			18								
469.60			19								
468.60			20								

Groundwater Data: Water after auger removal 24" below top of ground elevation.
Comments:



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BORING LOG

Sheet 1 of 1

Client: City of Ottawa
Project Name: Ottawa Industrial Park Unit 3
Project Site: Ottawa, Illinois

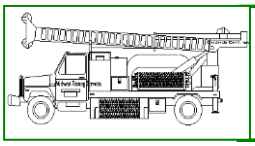
Boring No. SB-5
Surface Elev. 488.40
Auger Depth 3.5' Rotary Depth NA
Start Date 06/04/19 Finish Date 06/04/19

Location: Per Sketch

DRILLED BY: Randy Safranski

(DEPTH) ELEV. 488.40	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES: Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear	Moisture (%)	Dry Density (PCF)	REMARKS
487.40	Black Silty Clay Topsoil 8"		1								
486.40	Stiff Brownish Gray Silty Clay (CL)		2								First Water 486.4
485.40	Very Dense Brown/Gray Weathered Shale		3	1	SS	1.8	14	B	20		
484.40	Boring Terminated Auger Refusal		4								
483.40			5								
482.40			6								
481.40			7								
480.40			8								
479.40			9								
478.40			10								
477.40			11								
476.40			12								
475.40			13								
474.40			14								
473.40			15								
472.40			16								
471.40			17								
470.40			18								
469.40			19								
468.40			20								

Groundwater Data: Water after auger removal 24" below top of ground elevation.
Comments:



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BORING LOG

Sheet 1 of 1

Client: City of Ottawa
Project Name: Ottawa Industrial Park Unit 3
Project Site: Ottawa, Illinois

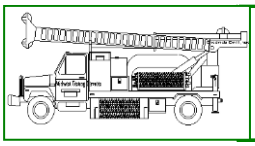
Boring No. SB-6
Surface Elev. 486.10
Auger Depth 6.5' Rotary Depth NA
Start Date 06/04/19 Finish Date 06/04/19

Location: Per Sketch

DRILLED BY: Randy Safranski

(DEPTH) ELEV.	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES: Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear	Moisture (%)	Dry Density (PCF)	REMARKS
486.10											
485.10	Black Silty Clay Topsoil 13"		1								First Water 485.3
484.10			2								
483.10	Stiff to Very Stiff Brownish Gray Silty Clay (CL)		3	1	SS	1.0	8	B	27		
482.10			4								
481.10			5	2	SS	2.6	17	B	29		
480.10	Very Dense Brown/Gray Weathered Shale		6								
479.10	Boring Terminated Auger Refusal		7								
478.10			8								
477.10			9								
476.10			10								
475.10			11								
474.10			12								
473.10			13								
472.10			14								
471.10			15								
470.10			16								
469.10			17								
468.10			18								
467.10			19								
466.10			20								

Groundwater Data: Water after auger removal 10" below top of ground elevation.
Comments:



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BORING LOG

Sheet 1 of 1

Client: City of Ottawa
Project Name: Ottawa Industrial Park Unit 3
Project Site: Ottawa, Illinois

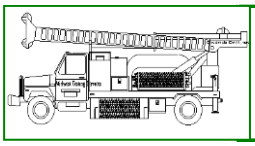
Boring No. SB-7
Surface Elev. 487.20
Auger Depth 3.5' Rotary Depth NA
Start Date 06/04/19 Finish Date 06/04/19

Location: Per Sketch

DRILLED BY: Randy Safranski

(DEPTH) ELEV. 487.20	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES: Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear	Moisture (%)	Dry Density (PCF)	REMARKS
486.20	Black Silty Clay Topsoil 12"		1								
485.20	Stiff Brownish Gray Silty Clay (CL)		2								
484.20	Very Dense Brown/Gray Weathered Shale		3	1	SS	4.2	30	B	16		First Water 484.2
483.20	Boring Terminated Auger Refusal		4								
482.20			5								
481.20			6								
480.20			7								
479.20			8								
478.20			9								
477.20			10								
476.20			11								
475.20			12								
474.20			13								
473.20			14								
472.20			15								
471.20			16								
470.20			17								
469.20			18								
468.20			19								
467.20			20								

Groundwater Data: Water after auger removal 36" below top of ground elevation.
Comments:



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BORING LOG

Sheet 1 of 1

Client: City of Ottawa
Project Name: Ottawa Industrial Park Unit 3
Project Site: Ottawa, Illinois

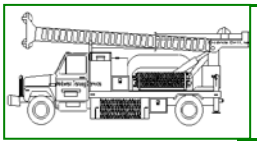
Boring No. SB-8
Surface Elev. 488.30
Auger Depth 4.5' Rotary Depth NA
Start Date 06/04/19 Finish Date 06/04/19

Location: Per Sketch

DRILLED BY: Randy Safranski

(DEPTH) ELEV. 488.30	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES: Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear	Moisture (%)	Dry Density (PCF)	REMARKS
487.30	Black Silty Clay Topsoil 13"		1								
486.30	Stiff Brownish Gray Silty Clay (CL)		2								
485.30			3	1	SS	1.3	11	B	29		First Water 485.3
484.30	Very Dense Brown/Gray Weathered Shale		4								
483.30	Boring Terminated Auger Refusal		5								
482.30			6								
481.30			7								
480.30			8								
479.30			9								
478.30			10								
477.30			11								
476.30			12								
475.30			13								
474.30			14								
473.30			15								
472.30			16								
471.30			17								
470.30			18								
469.30			19								
468.30			20								

Groundwater Data: Water after auger removal 36" below top of ground elevation.
Comments:



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BORING LOG

Sheet 1 of 1

Client: City of Ottawa
Project Name: Ottawa Industrial Park Unit 2
Project Site: Ottawa, Illinois

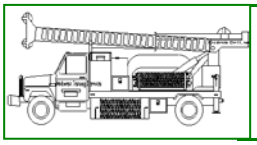
Boring No. B-16
Surface Elev. 0.00
Auger Depth 6' Rotary Depth NA
Start Date 02/13/12 Finish Date 02/13/12

Location: Per Sketch

DRILLED BY: Jeff Safranski, CME-55

(DEPTH) ELEV.	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES: Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear	Moisture (%)	Dry Density (PCF)	REMARKS
0.00											
-1.00	Topsoil 15"		1								
-2.00			2								
-3.00	Stiff Brownish Gray Clay		3	1	SS	1.6	8	B	21		
-4.00	Very Dense Limestone Caprock		4								
-5.00	Very Dense Brownish Gray Weathered Shale		5	2	SS	---	100/3"	---	---		
-6.00			6								
-7.00			7								
-8.00			8								
-9.00			9								
-10.00			10								
-11.00			11								
-12.00			12								
-13.00			13								
-14.00			14								
-15.00			15								
-16.00			16								
-17.00			17								
-18.00			18								
-19.00			19								
-20.00			20								

Groundwater Data: No groundwater encountered at time of subsurface investigation.
Comments:



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BORING LOG

Sheet 1 of 1

Client: City of Ottawa
Project Name: Ottawa Industrial Park Unit 2
Project Site: Ottawa, Illinois

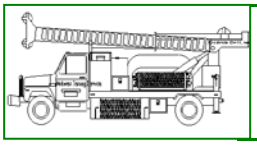
Boring No. B-17
Surface Elev. 0.00
Auger Depth 6' Rotary Depth NA
Start Date 02/13/12 Finish Date 02/13/12

Location: Per Sketch

DRILLED BY: Jeff Safranski, CME-55

(DEPTH) ELEV.	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES: Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear	Moisture (%)	Dry Density (PCF)	REMARKS
0.00											
-1.00	Topsoil 16"		1								
-2.00			2								
-3.00	Stiff Brownish Gray Clay		3	1	SS	1.7	8	B	21		
-4.00			4								
-5.00	Very Dense Brown Weathered Shale		5	2	SS	---	100/4"	---	---		
-6.00			6								
-7.00			7								
-8.00			8								
-9.00			9								
-10.00			10								
-11.00			11								
-12.00			12								
-13.00			13								
-14.00			14								
-15.00			15								
-16.00			16								
-17.00			17								
-18.00			18								
-19.00			19								
-20.00			20								

Groundwater Data: No groundwater encountered at time of subsurface investigation.
Comments:



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BORING LOG

Sheet 1 of 1

Client: City of Ottawa
Project Name: Ottawa Industrial Park Unit 2
Project Site: Ottawa, Illinois

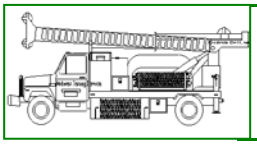
Boring No. B-18
Surface Elev. 0.00
Auger Depth 8.5' Rotary Depth NA
Start Date 02/13/12 Finish Date 02/13/12

Location: Per Sketch

DRILLED BY: Jeff Safranski, CME-55

(DEPTH) ELEV.	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES: Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear	Moisture (%)	Dry Density (PCF)	REMARKS
0.00											
-1.00	Topsoil 16"		1								
-2.00			2								
-3.00			3	1	SS	1.4	7	B	22		
-4.00	Stiff Brownish Gray Clay		4								
-5.00			5								
-6.00			6	2	SS	1.6	8	B	20		
-7.00			7								
-8.00	Very Dense Brown Weathered Shale		8	3	SS	---	100 / 5"	---	---		
-9.00			9								
-10.00			10								
-11.00			11								
-12.00			12								
-13.00			13								
-14.00			14								
-15.00			15								
-16.00			16								
-17.00			17								
-18.00			18								
-19.00			19								
-20.00			20								

Groundwater Data: No groundwater encountered at time of subsurface investigation.
Comments:



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BORING LOG

Sheet 1 of 1

Client: City of Ottawa
Project Name: Ottawa Industrial Park Unit 2
Project Site: Ottawa, Illinois

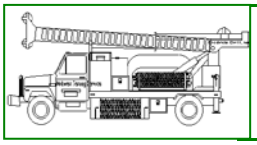
Boring No. B-19
Surface Elev. 0.00
Auger Depth 20' Rotary Depth NA
Start Date 02/13/12 Finish Date 02/13/12

Location: Per Sketch

DRILLED BY: Jeff Safranski, CME-55

(DEPTH) ELEV.	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES: Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear	Moisture (%)	Dry Density (PCF)	REMARKS
0.00											
-1.00	Black, Gray, Brown Silty Clay		1								
-2.00			2								
-3.00	Brown, Gray Weathered Shale		3	1	SS						
-4.00			4								
-5.00			5	2	SS						
-6.00			6								
-7.00			7								
-8.00			8	3	SS						
-9.00			9								
-10.00			10	4	SS						
-11.00	Gray Shale With Limestone Stringers		11								
-12.00			12								
-13.00			13	5	SS						
-14.00			14								
-15.00			15	6	SS						
-16.00			16								
-17.00			17								
-18.00			18	7	SS						
-19.00			19								
-20.00	Bottom of Boring		20								

Groundwater Data: No groundwater encountered at time of subsurface investigation.
Comments:



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BORING LOG

Sheet 1 of 1

Client: City of Ottawa
Project Name: Ottawa Industrial Park Unit 2
Project Site: Ottawa, Illinois

Boring No. B-20
Surface Elev. 0.00
Auger Depth 20' Rotary Depth NA
Start Date 02/13/12 Finish Date 02/13/12

Location: Per Sketch

DRILLED BY: Jeff Safranski, CME-55

(DEPTH) ELEV.	DESCRIPTION OF MATERIALS	Graphic Log	Depth in feet	SAMPLES: Sample No.	Sample Type	Qu (TSF)	N Value (Blows)	Bulge / Shear	Moisture (%)	Dry Density (PCF)	REMARKS
0.00											
-1.00	Black, Gray Silty Clay		1								
-2.00			2								
-3.00	Brown Weathered Shale		3	1	SS						
-4.00			4								
-5.00			5	2	SS						
-6.00			6								
-7.00			7								
-8.00			8	3	SS						
-9.00			9								
-10.00			10	4	SS						
-11.00	Gray Shale With Limestone Stringers		11								
-12.00			12								
-13.00			13	5	SS						
-14.00			14								
-15.00			15								
-16.00			16	6	SS						
-17.00			17								
-18.00			18	7	SS						
-19.00			19								
-20.00	Bottom of Boring		20								

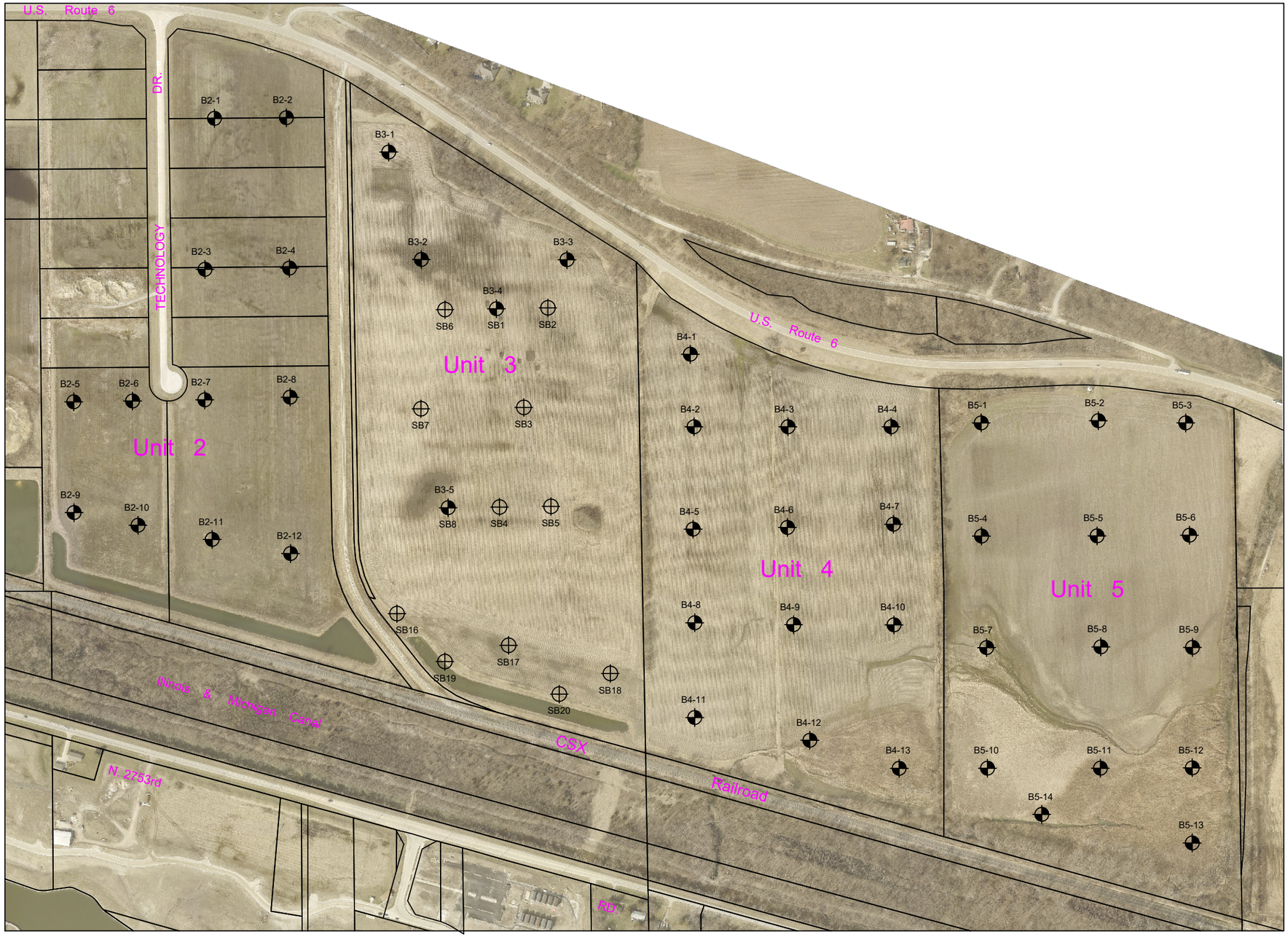
Groundwater Data: No groundwater encountered at time of subsurface investigation.
Comments:

N

0 500

(IN FEET)

SCALE 1" = 500'



Name	Northing	Easting	Elevation
B2–1	1,706,949.17	858,884.11	492.28
B2–2	1,706,952.09	859,209.20	492.37
B2–3	1,706,265.35	858,840.80	490.61
B2–4	1,706,270.92	859,223.42	490.57
B2–5	1,705,665.35	858,246.30	489.17
B2–6	1,705,669.12	858,512.51	489.46
B2–7	1,705,674.30	858,839.68	488.75
B2–8	1,705,686.63	859,226.38	488.94
B2–9	1,705,163.77	858,247.83	486.98
B2–10	1,705,106.61	858,537.20	486.92
B2–11	1,705,042.53	858,873.42	486.93
B2–12	1,704,978.29	859,228.23	487.04
B3–1	1,706,796.33	859,673.17	488.51
B3–2	1,706,310.01	859,820.88	485.33
B3–3	1,706,308.78	860,480.72	485.59
B3–4	1,706,087.00	860,161.00	485.69
B3–5	1,705,186.00	859,942.00	488.39
B4–1	1,705,880.81	861,039.23	486.84
B4–2	1,705,552.60	861,055.89	487.61
B4–3	1,705,552.60	861,481.43	487.50
B4–4	1,705,552.60	861,948.54	490.29
B4–5	1,705,088.75	861,051.71	487.77
B4–6	1,705,096.72	861,479.02	487.95
B4–7	1,705,111.06	861,958.95	490.33
B4–8	1,704,665.22	861,059.41	486.98
B4–9	1,704,656.30	861,507.00	487.55
B4–10	1,704,655.29	861,961.72	489.50
B4–11	1,704,236.16	861,059.14	487.11
B4–12	1,704,133.20	861,580.83	487.66
B4–13	1,704,006.03	861,985.27	488.99
B5–1	1,705,570.14	862,356.73	495.29
B5–2	1,705,580.03	862,887.07	501.20
B5–3	1,705,570.14	863,283.17	504.41
B5–4	1,705,057.56	862,358.00	493.39
B5–5	1,705,058.32	862,881.45	498.53
B5–6	1,705,061.70	863,299.99	502.07
B5–7	1,704,552.46	862,381.09	491.78
B5–8	1,704,556.19	862,898.89	495.70
B5–9	1,704,553.45	863,313.68	498.28
B5–10	1,704,006.30	862,385.16	491.99
B5–11	1,704,006.51	862,896.60	493.76
B5–12	1,704,008.21	863,312.51	497.04
B5–13	1,703,668.21	863,312.69	495.05
B5–14	1,703,797.56	862,631.01	492.33

Horizontal Datum is relative to Illinois State Plane Coordinate System, East Zone NAD83 (2011)

Elevations are existing ground relative to the North American Vertical Datum of 1988 (NAVD88).

NOTE:

GROUND SURFACE ELEVATIONS AT THE BORINGS WERE ACQUIRED BY ETSHEID, DUTTLINGER & ASSOCIATES, INC.

LEGEND

BORING LOCATION TSC L-99,961 OCT. 2025

MTS 2012 & 2019

BORING LOCATION PLAN
OTTAWA INDUSTRIAL PARK
US ROUTE 6, EAST OF ROUTE 71
OTTAWA, ILLINOIS

TESTING SERVICE CORPORATION
457 EAST GUNDERSEN DRIVE
CAROL STREAM, ILLINOIS 60188

DRAWN BY: FFE
CHECKED BY: AJB
JOB NO. : L–99,961
DATE: 11–07–25

PAGE NO.

1 OF 1